

§ 2.7 Linear Elasto Statics

i, j, k, l are indices associated with components. Repeated indices sum 1 to n_{sd}

As with heat transfer we will make two passes through the weak form -
 first: with indicial notation
 second: with vectors and matrices

The transition between the two more complex because we

- will collapse 2nd order tensor's to vectors and a 4th order tensor to a matrix
- Change the definition of shear strain (just by a factor of 2)

Quantities of interest

u_i - displacement components - u is now a vector \underline{u}

σ_{ij} - Cauchy stress tensor components

f_i - body force vector components

ϵ_{ij} - Green's strain tensor components

$$\epsilon_{ij} \equiv u_{(i,j)} \equiv \frac{1}{2}(u_{i,j} + u_{j,i})$$

symmetric derivative

The stresses and strains are related through material parameters -
 Generalized Hooke's Law (Constitutive eq.)

$$\sigma_{ij} = C_{ijkl} \epsilon_{kl}$$

By definition $\epsilon_{kl} = \epsilon_{lk}$ symmetric

By balance of angular momentum
 (equilibrium of stress block)

$$\sigma_{ij} = \sigma_{ji} \text{ symmetric}$$

This leads to symmetries in C_{ijkl}

$$C_{ijkl} = C_{klij}$$

$$C_{ijkl} = C_{jikl}$$

$$C_{ijkl} = C_{ijlk}$$

C_{ijkl} must also be pos. def. (at least for what we do)

$$C_{ijkl} \psi_{ij} \psi_{kl} \geq 0$$

$$C_{ijkl} \psi_{ij} \psi_{kl} = 0 \text{ iff } \psi_{ij} = \emptyset$$

all terms zero

Strong form

Given $f_i : \Omega \rightarrow \mathbb{R}$, $g_i : \Gamma_{g_i} \rightarrow \mathbb{R}$ and $h_i : \Gamma_{h_i} \rightarrow \mathbb{R}_g$
 find $u_i : \Omega \rightarrow \mathbb{R}$ such that

$$\nabla_{ij} u_i + f_i = 0 \quad \text{in } \Omega \quad (\text{equilibrium})$$

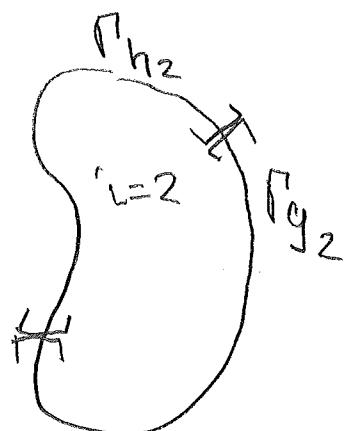
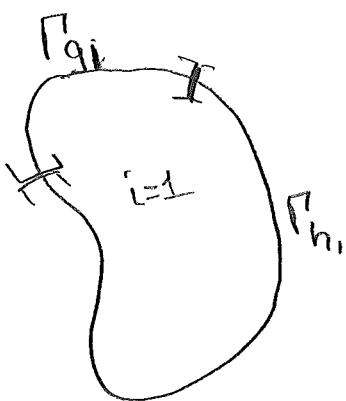
$$u_i = g_i \quad \text{on } \Gamma_{g_i} \quad \text{displacement B.C.}$$

$$\sigma_{ij} n_j = h_i \quad \text{traction B.C.}$$

where

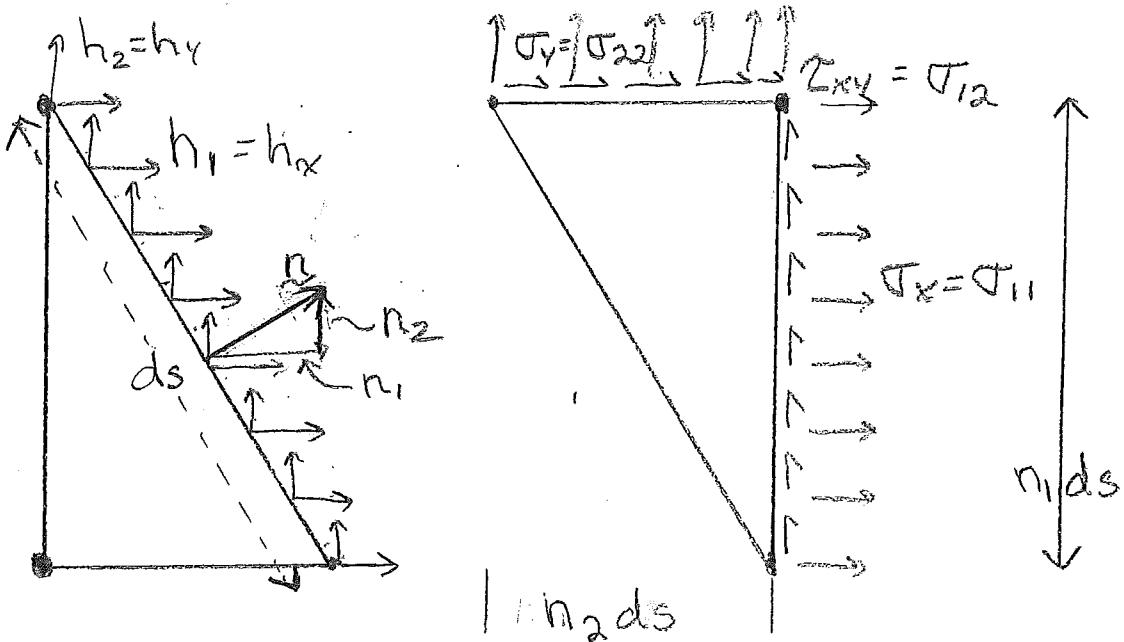
$$\epsilon_{ij} = u_{(i,j)} = \frac{u_{i,j} + u_{j,i}}{2}$$

$$\sigma_{ij} = C_{ijkl} \epsilon_{kl}$$



$$\Gamma = \overline{\Gamma}_{g_i} \cup \overline{\Gamma}_{h_i} \quad i=1(1)n_{sd}$$

$$\Omega = \Gamma_{g_i} \cap \Gamma_{h_i} \quad i=1(1)n_{sd}$$



To see expressions for h_1 and h_2
write statical equivalents

$$\sum F_x = 0$$

$$h_1 ds = \sigma_{11} n_1 ds + \sigma_{12} n_2 ds$$

$$h_1 = \sigma_{11} n_1 + \sigma_{12} n_2$$

$$\sum F_y = 0$$

$$h_2 ds = \sigma_{22} n_2 ds + \sigma_{12} n_1 ds$$

$$h_2 = \sigma_{22} n_2 + \sigma_{12} n_1$$

Applying MWR

$$\int_{\Omega} w_i(\nabla_{ij} f_j - f_i) d\Omega = 0 \quad \text{see Lemma 2 on pg 79 of text}$$

integrate first term by parts

$$-\int_{\Omega} w_i(\nabla_{ij} f_j) d\Omega + \int_{\Gamma} w \nabla_{ij} f_j d\Gamma - \int_{\Omega} w_i f_i d\Omega = 0$$

Because of symmetry of $\nabla_{ij} f_j$

$$\int_{\Gamma} w \nabla_{ij} f_j d\Gamma = \int_{\Gamma_{h_1}} w \nabla_{ij} f_j d\Gamma + \int_{\Gamma_{h_2}} w \nabla_{ij} f_j d\Gamma \quad \text{for } w \in V$$

$$= \int_{\Gamma_{h_1}} w_1 \nabla_{1j} n_j d\Gamma + \int_{\Gamma_{h_2}} w_2 \nabla_{2j} n_j d\Gamma \quad (n_{sd}=2)$$

since $\Gamma_{h_1} \neq \Gamma_{h_2}$ in general - have two terms

Another way to write it is

$$\sum_{\alpha=1}^{n_{sd}} \left(\int_{\Gamma_{h_\alpha}} w_\alpha \nabla_{\alpha j} n_j d\Gamma \right) = \sum_{\alpha=1}^{n_{sd}} \left(\int_{\Gamma_{h_\alpha}} w_\alpha h_\alpha d\Gamma \right)$$

no sum on α

Putting it together we can write the weak form -

Given $f_i: \Omega \rightarrow \mathbb{R}$, $g_i: \Gamma_{g_i} \rightarrow \mathbb{R}$ and $h_i: \Gamma_{h_i} \rightarrow \mathbb{R}$, find $u_i \in S_i$ such that

$$\int_{\Omega} w_{(i,j)} T_{ij} d\Omega = \int_{\Omega} w_i f_i d\Omega + \sum_{\alpha=1}^{n_{sd}} \left(\int_{\Gamma_{h_i}} w_{\alpha} h_{\alpha} d\Gamma \right) \quad \text{with } \sum_{\alpha=1}^{n_{sd}} w_{\alpha} \text{ no sum in integral}$$

with $T_{ij} = C_{ijkl} \epsilon_{ke}$, $\epsilon_{kl} = u_{(i,j)} = \frac{u_{i,j} + u_{j,i}}{2}$

$$S_i = \{u_i \mid u_i \in H^1, u_i = g_i \text{ on } \Gamma_{g_i}\} \quad i = 1(1)n_{sd}$$

$$D_i = \{w_i \mid w_i \in H^1, w_i = 0 \text{ on } \Gamma_{g_i}\} \quad i = 1(1)n_{sd}$$

1. Review text discussion of $(S) \rightarrow (\mathcal{W}), (\mathcal{W}) \rightarrow (S)$

note - $\int_{\Omega} w_{(i,j)} T_{ij} d\Omega = \int_{\Omega} w_{(i,j)} C_{ijkl} \epsilon_{ke} d\Omega$

Stated with abstract form -

Given f, g and h , (defined over right stuff)

find $\tilde{u} \in \mathcal{S}$ such that

$$a(\tilde{w}, \tilde{u}) = (\tilde{w}, f) + (\tilde{w}, h)_{\Gamma} \quad \forall \tilde{w} \in \mathcal{W}$$

where

$$a(\tilde{w}, \tilde{u}) = \int_{\Omega} w_{(i,j)} C_{ijkl} \epsilon_{ke} d\Omega$$

$$(\tilde{w}, f) = \int_{\Omega} w_i f_i d\Omega$$

$$(\tilde{w}, h)_{\Gamma} = \sum_{\alpha=1}^{n_{sd}} \int_{\Gamma_{h_i}} w_{\alpha} h_{\alpha} d\Gamma \quad \text{no sum.}$$

We now work toward a "convenient" matrix description

we begin by representing the necessary terms from our second order tensors as terms listed in a vector-

(they still are second order tensors and transform coordinates like second order tensors - (not like vectors))

recalling that $T_{ij} = T_{ji}$

for $N_{sd}=2$ $T_{12} = T_{21}$, so only 3 independent terms = T_{11}, T_{22} and T_{12} ($\neq T_{21}$)

Thus we will define

$$N_{sd}=2 \quad \underline{T} = \begin{Bmatrix} T_{11} \\ T_{22} \\ T_{12} \end{Bmatrix}$$

for $N_{sd}=3$ we have $T_{12} = T_{21}, T_{13} = T_{31}, T_{23} = T_{32}$
so $T_{11}, T_{22}, T_{33}, T_{12}, T_{13} \neq T_{23}$ can be the 6 independent terms

The text book ordering of the 6 terms

$$N_{sd}=3 \quad \underline{T} = \begin{Bmatrix} T_{11} \\ T_{22} \\ T_{33} \\ T_{23} \\ T_{13} \\ T_{12} \end{Bmatrix}$$

Like wise we define a 3×1 strain vector for $n_{sd}=2$ and 6×1 strain vector for 3D

How we convert from Green's strains to "Engineering strains" (double "shear" terms).

$$n_{sd}=2 \quad \underline{\underline{\epsilon}}(u) = \{ \underline{\epsilon}_I(u) \} = \begin{Bmatrix} u_{1,1} \\ u_{2,2} \\ u_{1,2} + u_{2,1} \end{Bmatrix}$$

$$n_{sd}=3 \quad \underline{\underline{\epsilon}}(u) = \{ \underline{\epsilon}_I(u) \} = \begin{Bmatrix} u_{1,1} \\ u_{2,2} \\ u_{3,3} \\ u_{2,3} + u_{3,2} \\ u_{1,3} + u_{3,1} \\ u_{1,2} + u_{2,1} \end{Bmatrix}$$

We need our constitutive eq. in terms of the new stress and strain vectors

$$\underline{\underline{\sigma}} = D \underline{\underline{\epsilon}}(u) \quad , \quad \{ \underline{\sigma} \}_{ex1}^2 = [D] \{ \underline{\epsilon}(u) \}_{ex1}^2$$

$$[D] = [D]^T \quad \text{symmetric} \quad e = n_{sd}(n_{sd}+1)/2, \quad e = 3 \text{ for } n_{sd}=2, \quad e = 6 \text{ for } n_{sd}=3$$

If we are given the terms C_{ijkl} using the known symmetries of C_{ijkl} and D as well as the relations from σ_{ij} to $\underline{\underline{\sigma}}$ and ϵ_{ij} to $\underline{\underline{\epsilon}}(u)$ we can construct terms in D .

Text book shows how to do this

Since no one does this - I will not cover it or ask you to do it on home work or test.

with all this done we have

$$\omega_{ijkl} c_{ijkl} u_{kl} = \underbrace{\epsilon(\omega)^T}_{\Omega} \underbrace{\epsilon(u)}_v$$

$$a(\omega, u) = \int_{\Omega} \underbrace{\epsilon(\omega)^T}_{\Omega} \underbrace{\epsilon(u)}_v d\Omega$$

Doing our normal decomposition of
 $\underline{u}^h = \underline{v}^h + \underline{g}^h$, $\underline{u}^h \in \underline{\delta}^h$, $\underline{v}^h \in \underline{\nu}^h$, $\underline{g}^h \in \underline{\gamma}^h$

where:

$$\underline{\nu}^h = \{ \underline{w}^h \mid w_i^h \in \nu_i^h \}, \quad \nu_i^h = \{ w_i^h \mid w_i^h \in H^1, w_i^h = 0 \text{ on } \Gamma_{g_i} \}$$

$$\underline{\delta}^h = \{ \underline{u}^h \mid u_i^h \in \delta_i^h \}, \quad \delta_i^h = \{ u_i^h \mid u_i^h \in H^1, u_i^h = g_i \text{ on } \Gamma_{g_i} \}$$

we have our Galerkin form

Given \underline{f} , \underline{g} and \underline{h} as before find $\underline{u}^h = \underline{v}^h + \underline{g}^h \in \underline{\delta}^h$
such that

$$a(\underline{w}^h, \underline{v}^h) = (\underline{w}^h, \underline{f}) + (\underline{w}^h, \underline{h})_P - a(\underline{w}^h, \underline{g}^h)$$

To get to the matrix form we have
to deal with the fact that essential BC can
be associated with any nodes (in
general dof holders)

($\text{ndof}^2 \text{ nodes}^2$
for system)

Assuming nodal dof only where each
node can have n_{dof} ,
lets use $n_{dof} = n_{sd}$ for the basic vector system

in this case the total possible number of dof is

$$n_{DOF} = n_{dof} n_{np} = n_{sd} n_{np} \text{ for our current case}$$

of nodes

n_{eg} = actual # dof in global system

$n_{eg} < n_{DOF} \Leftarrow$ must have sufficient essential B.C.

Consider again the set $\mathcal{N} = \{1, 2, 3, \dots, n_{np}\}$
and now define set

$$\mathcal{N}_{g_i} = \{\text{nodes with a } g_i\}$$

with this we can write

$$v_i^n = \sum_{A \in \mathcal{N} - \mathcal{N}_{g_i}} N_A d_{IA} \quad i=1(1)n_{sd}$$

$$g_i = \sum_{A \in \mathcal{N}_{g_i}} N_A g_{IA} \quad i=1(1)n_{sd}$$

Introducing yet another construct:
Euclidean basis vector e_i

$$n_{sd}=2 \quad e_1 = \begin{cases} 1 \\ 0 \end{cases}, \quad e_2 = \begin{cases} 0 \\ 1 \end{cases}$$

$$n_{sd}=3 \quad e_1 = \begin{cases} 1 \\ 0 \\ 0 \end{cases}, \quad e_2 = \begin{cases} 0 \\ 1 \\ 0 \end{cases}, \quad e_3 = \begin{cases} 0 \\ 0 \\ 1 \end{cases}$$

with this we can write

$$\underline{\underline{w}}^h = \underline{U}_i^h \underline{e}_i, \quad \underline{\underline{g}}^h = \underline{G}_i^h \underline{e}_i \quad (\text{removing sum})$$

$$\underline{\underline{w}}^h = \underline{W}_i^h \underline{e}_i, \quad \underline{W}_i^h = \sum_{A \in \mathcal{N} - \mathcal{N}_{g_i}} N_A C_{iA}$$

Note - notation still a bit messy but idea is to include the correct terms in each summation as well as accounting for components -

Substituting all this in and recalling the C_{iA} 's are arbitrary we have

$$\sum_{j=1}^{n_{sd}} \left(\sum_{B \in \mathcal{N} - \mathcal{N}_{g_j}} a(N_A \underline{e}_i, N_B \underline{e}_j) d_{jB} \right) =$$

$$\begin{aligned} & \text{(Using the } \\ & \text{Dof} = n_{sd} \\ & \text{case)} & (N_A \underline{e}_i, f) + (N_A \underline{e}_i, h) \\ & - \sum_{j=1}^{n_{sd}} \left(\sum_{B \in \mathcal{N} - \mathcal{N}_{g_j}} a(N_A \underline{e}_i, N_B \underline{e}_j) g_{jB} \right) & A \in \mathcal{N} - \mathcal{N}_{g_i} \\ & & 1 \leq i \leq n_{sd} \end{aligned}$$

for writing the final matrix form

we can use the ID device - now as a matrix

$$ID(i, A)$$

$$\begin{aligned} i &= 1(1) n_{sd} && \text{(vector system)} \\ A &= 1(1) n_{np} && \text{(nodal dof only)} \end{aligned}$$

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P=0
Do A = 1 to Nnp
    Do i = 1 to Nsd
        If  $\tilde{A} \in \mathcal{N} - \mathcal{N}_{g_i}$ 
            P=P+1
            ID(i, A)=P   ← equation # in R
        else
            ID(i, A)=0
        end if
    end Do
end Do
Neq=P ← total number of actual equations

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Using this we have

$$\{K\}\{\tilde{d}\} = \{F\} \quad (K\tilde{d} = F)$$

$$P = ID(i, A), \quad Q = ID(j, B)$$

$$K_{PQ} = a(N_A \tilde{e}_i, N_B \tilde{e}_j)$$

$$F_P = (N_A \tilde{e}_i, f) + (N_A \tilde{e}_i, b)_P - \sum_{j=1}^{Nsd} \left(\sum_{B \in \mathcal{N}_{g_j}} a(N_A \tilde{e}_i, N_B \tilde{e}_j) g_{jB} \right)$$

⇒ We will see exactly how terms go into place when we look at our pseudo code later

To complete the ability to use vector matrix form we want to introduce the "B" matrix again

$$\underline{\epsilon}(\underline{N_A} \underline{\epsilon_i}) = \underline{B}_A \underline{\epsilon_i} \leqslant \text{Strain-displacement operator}$$

for $n_{sd}=2$

$$\underline{B}_A = \begin{bmatrix} N_{A,1} & 0 \\ 0 & N_{A,2} \\ N_{A,2} & N_{A,1} \end{bmatrix}$$

$\overset{3 \times 2}{\uparrow}$
#strain components

for $n_{sd}=3$

$$\begin{bmatrix} N_{A,1} & 0 & 0 \\ 0 & N_{A,2} & 0 \\ 0 & 0 & N_{A,3} \\ 0 & N_{A,3} & N_{A,2} \\ N_{A,3} & 0 & N_{A,1} \\ N_{A,2} & N_{A,1} & 0 \end{bmatrix}$$

6×3

With this we can write (not very useful!)

$$K_{PQ} = \underline{\epsilon_i}^T \left\{ \underline{B}_A^T D \underline{B}_B d\Omega \right\} \underline{\epsilon_j}$$

and noting that $\int_R N_A \underline{\epsilon_i}^T d\Omega = \int_R N_A f_i d\Omega$

$$\int_R N_A \underline{\epsilon_i} h d\Omega = \int_{\Gamma_{hi}} N_A h_i d\Gamma$$

$$F_p = \int_{\Omega} N_A f_i d\Gamma + \int_{\Gamma_{hi}} N_A h_i d\Gamma - \sum_{j=1}^{n_{sd}} \left(\sum_{\substack{A \in \mathcal{N}_B \\ B \in \mathcal{N}_G}} a(N_A e_i, N_B e_j) q_{jB} \right)$$

still messy -

Is $K_{PQ} = K_{QP}$ (symmetric)

$$K_{PQ} = \left(\int_{\Omega} e_i^T \left(\int_{\Omega} B_A^T D B_B d\Omega \right) e_j \right)^T \leftarrow \begin{array}{l} \text{since its} \\ \text{"just a" number} \end{array}$$

$$= e_j^T \left(\int_{\Omega} B_B^T D^T B_A e_i \right) \quad \text{with } D = D^T$$

$$= e_j^T \left(\int_{\Omega} B_B^T D B_A e_i \right) \equiv K_{QP}$$

Positive-Definiteness of K

$$\zeta^T K \zeta \geq 0, \zeta^T K \zeta = 0 \text{ iff } \zeta = \emptyset$$

$$\zeta^T K \zeta = \sum_{\substack{P, Q=1 \\ \text{neg}}} C_P K_{PQ} C_Q$$

$$= \sum_{i,j=1}^{n_{dof}} \left(\sum_{\substack{A \in \mathcal{N}_B \\ B \in \mathcal{N}_G}} C_{ia} a(N_A e_i, N_B e_j) C_{jb} \right)$$

use bilinearity

$$= a \left(\sum_{i=1}^{n_{dof}} \left(\sum_{\substack{A \in \mathcal{N}_B \\ B \in \mathcal{N}_G}} C_{ia} N_A e_i \right), \sum_{j=1}^{n_{dof}} \left(\sum_{\substack{B \in \mathcal{N}_B \\ G \in \mathcal{N}_G}} C_{jb} N_B e_j \right) \right)$$

$$= a(\omega^h, \omega^h) = \int_{\Omega} w_{(ij)}^h C_{ijk} w_{(k,e)}^h d\Omega$$

$\zeta \geq 0 \text{ for posdef } C_{ijk}$

we have positive ness

$$\text{assume } \underbrace{\mathbf{C}^T}_{\sim} \underbrace{\mathbf{R}}_{\sim} \underbrace{\mathbf{C}}_{\sim} = 0$$

That requires $\omega_{(ij)}^h C_{ijk\ell} \omega_{(\ell k)}^h = 0$

This requires $\omega_{(ij)}^h = 0$ for posdef. $C_{ijk\ell}$

There are a limited set of $\omega_{(ij)}^h \neq 0$ for which $\omega_{(ij)}^h = \frac{\omega_{ij}^h + \omega_{ji}^h}{2} = 0$

The correspond

$$\text{for } n_{sd}=2 \quad \underbrace{\omega_{(ij)}^h(x)}_{\sim} = \underbrace{c_1}_{\sim} + c_3 \underbrace{(x_1 e_2 - x_2 e_1)}_{\sim} \quad \begin{matrix} \text{rigid body rotation in plane} \\ \text{rigid body translation} \end{matrix}$$

$$\text{for } n_{sd}=3 \quad \underbrace{\omega_{(ij)}^h(x)}_{\sim} = \underbrace{c_1}_{\sim} + \underbrace{c_2}_{\sim} \times \underbrace{x}_{\sim} \quad \begin{matrix} \leftarrow \text{Rigid body} \\ \text{cross product} \end{matrix} \quad \begin{matrix} \text{rotations} \\ \text{rigid body translation} \end{matrix}$$

K is pos.-def. so long as
rigid body motions are constrained
by the prescribed $\underbrace{g^h}_{\sim}$

Finally - We do not want to do those crazy integrals to directly get K_{PQ} and F_P terms

We want to do elements - the whole element - at once and have the assembly operator take care of putting things in the right place the right way

$$\underline{\underline{K}} = \sum_{e=1}^{N_{el}} \underline{\underline{k}}^e, \quad \underline{\underline{F}} = \sum_{e=1}^{N_{el}} (\underline{\underline{k}}^e \underline{\underline{f}}^e)$$

N_{en} = # nodes / element

N_{ed} = # dof / node = N_{sd} for simple vector systems

$N_{ee} = N_{en} N_{ed} = \# \text{ of dof of the element}$
 \rightarrow will be the size of element $\underline{\underline{k}}^e, \underline{\underline{f}}^e$

$$\underline{\underline{k}}^e = \left[k_{pq}^e \right], \quad \underline{\underline{f}}^e = \left\{ f^{eq} \right\} \quad 1 \leq p, q \leq N_{ee}$$

$$k_{pq} = \int_{R_e} B_a^T D B_b d\alpha e_j^p \quad p = N_{ed}(a-1) + i \\ g = N_{ed}(b-1) + j$$

for $N_{sd} = 2$

$$\underline{\underline{B}}_a = \begin{bmatrix} N_{a,1} & 0 \\ 0 & N_{a,2} \\ N_{a,2} & N_{a,1} \end{bmatrix}$$

for
 $N_{sd} = 3$ $\underline{\underline{B}}_a =$

$$\begin{bmatrix} N_{a,1} & 0 & 0 \\ 0 & N_{a,2} & 0 \\ 0 & 0 & N_{a,3} \\ 0 & N_{a,3} & N_{a,2} \\ N_{a,3} & 0 & N_{a,1} \\ N_{a,2} & N_{a,1} & 0 \end{bmatrix}$$

what I called \hat{f}_p^e

$$\hat{f}_p^e = \int_{\Gamma_e} N_a f_i dr + \int_{\Gamma_e} N_a h_i d\Gamma - \sum_{g=1}^{N_{ee}} k_{pg} g_g^e$$

$$\Gamma_{hi}^e = \Gamma_{hi} \cap \Gamma^e$$

portion of elements

$g_g = g_j(\tilde{x}_b^e)$ if node is on Γ_g
is zero otherwise

In actual programs we avoid the e_i 's
and just do the whole thing at once
for example -

Define $B = [B_1, B_2, \dots, B_{nen}]$

then

$$\hat{K}^e = \int_{\Gamma_e} B^T D B dr$$

Stress evaluation $\hat{E}(u)$ = $\sum_{a=1}^{nen} B_a d_a$ dof at a node

$$\hat{\sigma}(x) = D(x) \hat{E}(u(x)) = D(x) \sum_{a=1}^{nen} B_a d_a^e$$

$$= D(x) B d^e$$

where

$$B = [B_1, B_2, B_3, \dots, B_{nen}], d = \begin{Bmatrix} d_1^e \\ d_2^e \\ \vdots \\ d_n^e \end{Bmatrix}$$

Before seeing how this all fits together in the form of pseudo code to a very basic F.E. code, we need one more tool - Numerical Integration (will cover Chapt. 3)

$$\underline{k}^e = \int_{\Omega} \underline{B}^T \underline{D} \underline{B} d\Omega \quad \text{is often messy because of mappings -}$$

Thus we will apply numerical integration

$$\underline{k}^e = \sum_{l=1}^{N_{int}} w_l \underline{B}^T(\underline{x}_l) \underline{D}(\underline{x}_l) \underline{B}(\underline{x}_l) + R^{int}$$

Since we do not know R^{int} past being able to say it small enough when we use a sufficient number of properly selected integration points, we will drop it and use

$$\underline{k}^e \approx \sum_{l=1}^{N_{int}} w_l \underline{B}^T(\underline{x}_l) \underline{D}(\underline{x}_l) \underline{B}(\underline{x}_l)$$